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Fading regularization method and DKQ plate elements for the Cauchy problem associated with the biharmonic equation

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Motivation: Inverse problems in thin plate bending

Thin plates find applications in several fields of science and engineering. We recall here the case of plate elements for a bridge deck. In some situations, it may be mandatory to ensure the embedding and support of these plates for the correct installation of this structure. However, direct measurements may be impossible or difficult to obtain during construction or during maintenance. An inverse problem then arises!

Objectives: The aim of this study is to solve the inverse Cauchy-type problem for the bilaplacian operator, which governs the bending of thin plates, such as boundary conditions are available only on part of the boundary. We pro- canambridges.com pose to apply the fading regularization method and to implement it numerically using Discrete Kirchhoff (DK) plate elements.



Figure 1: Plate elements for a bridge deck

The fading regularization method

• Idea: Find the solution of the equilibrium equation that best fits the boundary data while maintaining independence of the regularization parameter and stability towards noisy data

$$\mathbf{V}^{k+1} = \underset{\mathbf{V} \in H(\Gamma)}{\operatorname{Argmin}} \ \left\{ \|\mathbf{V} - \boldsymbol{\Phi}_d\|_{H(\Gamma_d)}^2 + c \|\mathbf{V} - \mathbf{V}^k\|_{H(\Gamma)}^2 \right\}$$

• The fading regularization algorithm:

$$\begin{vmatrix} \mathbf{V}^{k+1} = \operatorname{Argmin}_{\mathbf{V} \in \mathbb{R}^{5N}} J_c^{k+1}(\mathbf{V}) \\ \mathbf{V} \in \mathbb{R}^{5N} \end{vmatrix}$$
 with $\mathbf{V} = (d, \mathcal{M}_n, \mathcal{V}_n) = (W, \theta_{,x}, \theta_{,y}, \mathcal{M}_n, \mathcal{V}_n)$ under the equality constraints $\mathcal{E}(\mathbf{V}) = 0$

where

$$J_{c}^{k+1}(\mathbf{V}) = \|W_{|\Gamma_{d}} - \phi_{d}\|_{L^{2}(\Gamma_{d})}^{2} + \|n_{y}\theta_{,x} + n_{x}\theta_{,y}\|_{\Gamma_{d}} - \mu_{d}\|_{L^{2}(\Gamma_{d})}^{2} + \|\mathcal{M}_{n|\Gamma_{d}} - \mathcal{M}_{d}\|_{L^{2}(\Gamma_{d})}^{2} + \|\mathcal{V}_{n|\Gamma_{d}} - \mathcal{V}_{d}\|_{L^{2}(\Gamma_{d})}^{2} + c\Big(\|W - W^{k}\|_{L^{2}(\Gamma)}^{2} + \|\theta_{,x} - \theta_{,x}^{k}\|_{L^{2}(\Gamma)}^{2} + \|\theta_{,y} - \theta_{,y}^{k}\|_{L^{2}(\Gamma)}^{2} + \|\mathcal{M}_{n} - \mathcal{M}_{n}^{k}\|_{L^{2}(\Gamma)}^{2} + \|\mathcal{V}_{n} - \mathcal{V}_{n}^{k}\|_{L^{2}(\Gamma)}^{2}\Big)$$

Ill-posed Cauchy problem associated with the bilaplacian

The constitutive equation for the bending plate problem, under *Kirchhoff-Love* theory, is:

$$\int_{\Omega} \underbrace{\left((\mathbf{L} \nabla)^{t} \mathbf{D} (\mathbf{L} \nabla) w \right)}_{D\Delta^{2}w} \delta w \, dx dy = \int_{\Omega} q(x, y) \delta \omega \, dx dy
+ \int_{\Gamma} \left[\mathcal{M}_{n} \frac{\partial \delta w}{\partial n} - \mathcal{V}_{n} \delta w \right] \, ds + \sum_{i} \delta w_{i} R_{i}.$$

It is characterized by the biharmonic operator to which boundary conditions, related to the transverse displacement w, the normal rotation $\frac{\partial w}{\partial n}$, the moment \mathcal{M}_n and the shear force \mathcal{V}_n , are added at any point of the boundary.

$$\mathcal{M}_{n} = -D \left[\Delta w + (1 - \nu) \left(2n_{x}n_{y} \frac{\partial^{2}w}{\partial x \partial y} - n_{y}^{2} \frac{\partial^{2}w}{\partial x^{2}} - n_{x}^{2} \frac{\partial^{2}w}{\partial y^{2}} \right) \right]$$

$$\mathcal{V}_{n} = -D \left[\frac{\partial \Delta w}{\partial n} + (1 - \nu) \frac{\partial}{\partial s} \left[n_{x}n_{y} \left(\frac{\partial^{2}w}{\partial y^{2}} - \frac{\partial^{2}w}{\partial x^{2}} \right) + (n_{x}^{2} - n_{y}^{2}) \frac{\partial^{2}w}{\partial x \partial y} \right] \right]$$

These boundary conditions are not always accessible to measurements. Hence, in the case of an inaccessible part Γ_i of the boundary and an over-specified part of the boundary Γ_d , the resulting Cauchy problem is ill-posed in the sense of Hadamard.

$$\begin{cases} \Delta^2 w = 0 & \forall x \in \Omega \\ w = \varphi_d & \forall x \in \Gamma_d \\ w_{,n} = \psi_d & \forall x \in \Gamma_d \\ \mathcal{M}_n = \mathcal{M}_d & \forall x \in \Gamma_d \\ \mathcal{V}_n = \mathcal{V}_d & \forall x \in \Gamma_d \end{cases}$$

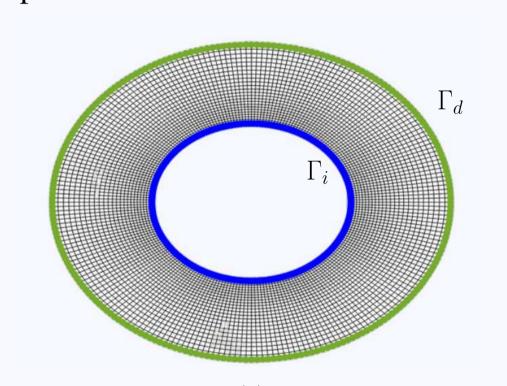
 $\partial \Omega = \Gamma_d \cup \Gamma_i$ et $\Gamma_d \cap \Gamma_i = \emptyset$. No boundary condition is given on the part Γ_i !

Numerical simulations

We consider the biharmonic analytical solution:

$$u_{an}(x,y) = \frac{1}{2}x(\sin x \cosh y - \cos x \sinh y), \quad (x,y) \in \Omega,$$

where Ω is a two-dimensional domain such as Cauchy data are given only on a part Γ_d of its boundary Γ . Two example of domains are considered:



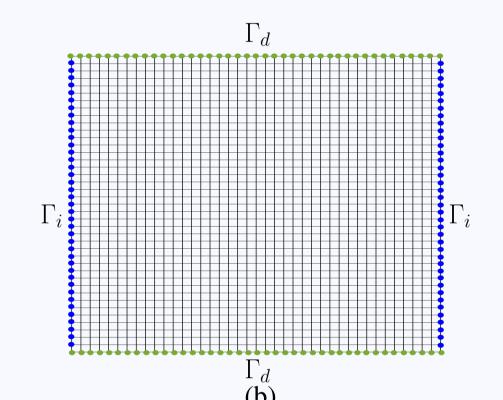
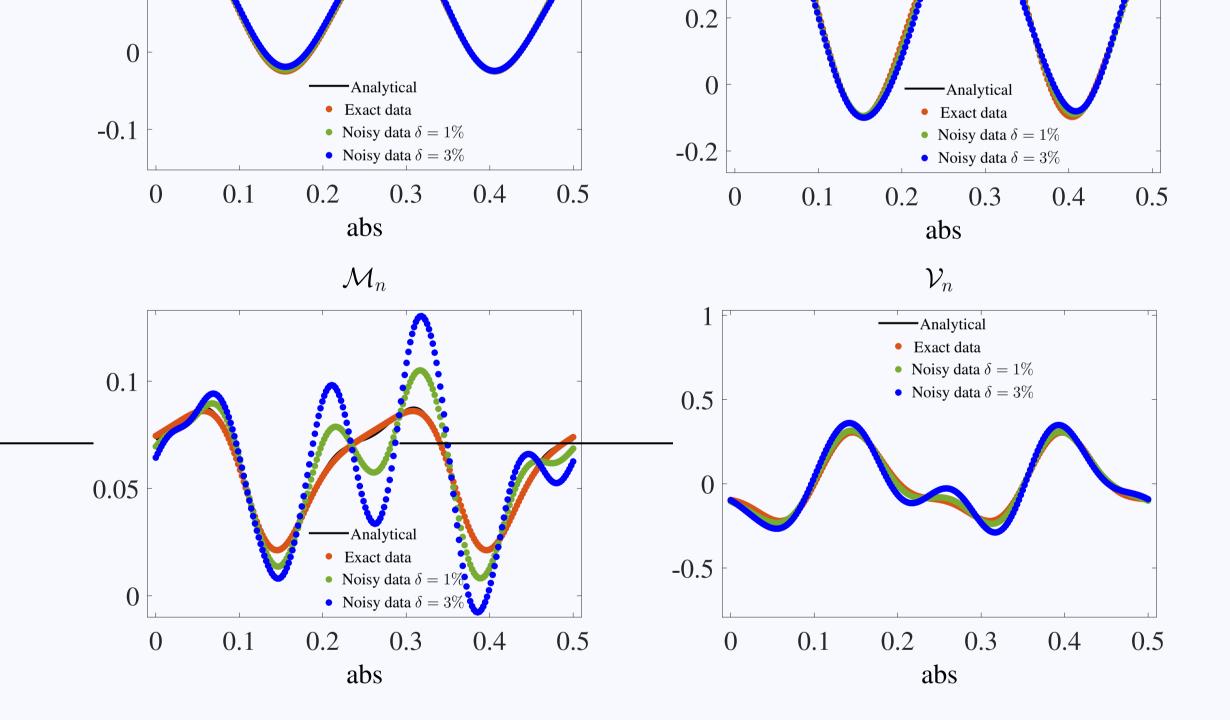
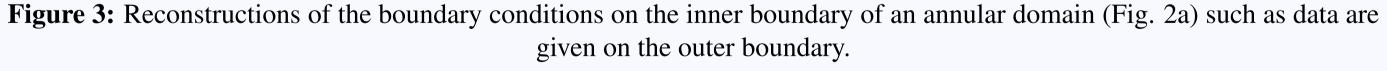


Figure 2: (a) An annular shaped domain and (b) a square domain such as data are given only on the part Γ_d of the boundary.





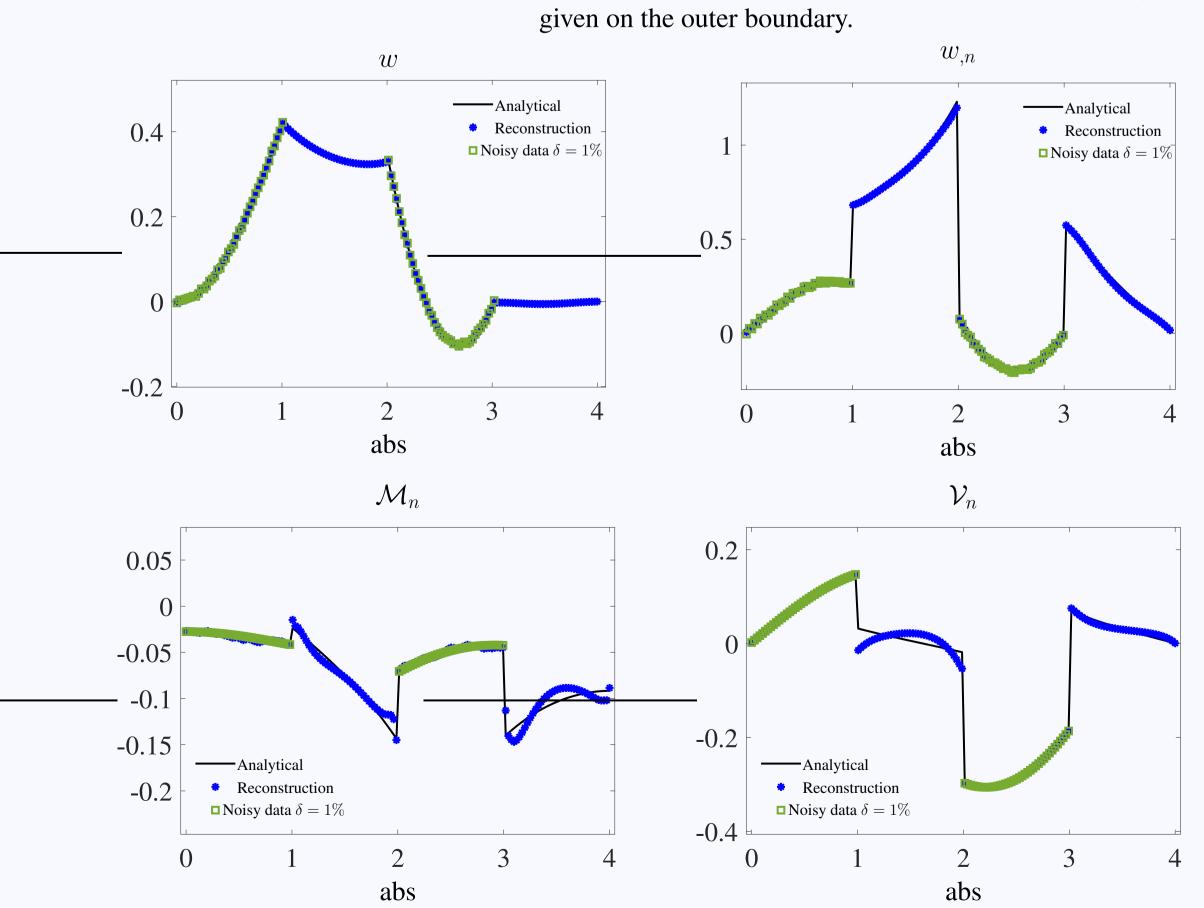


Figure 4: Reconstructions of the boundary conditions on the boundary of a square domain (Fig. 2b) such as data are given on two opposite sides.

Discrete Kirchhoff plate element

Discrete Kirchhoff Thin plates **Kirchhoff-Love theory**

 \mathcal{C}^1 regularity for the displacement w

Example of non--conforming elements : -Adini 1960 **-Melosh 1963** -Bazeley 1966

-Cheung 1968

Poor performance due to \mathcal{C}^{\perp} regularity issue when rotation field interpolation comes from displacement field

3 degrees of freedom per node:

Get rid of C^1 continuity in a discrete way.

 \mathcal{C}^0 regularity for w and independent interpolation for the rotation field

Thick plates

Discrete Kirchhoff

elements:

-MITC4

-DKMQ

-DSQ

Reissner-Mindlin theory

Example of thick plate

Neglecting the shear force by applying discrete Kirchhoff's theory (at corner nodes and element edges)

Discrete Kirchhoff (DK)

finite elements

Plate finite elements

Example of DK finite l elements : - **DKT Dhatt 1968 DKT Batoz 1980** DKQ Batoz 1982

- MKQ12 2017

The stiffness matrix of the DKQ element is defined as

$$\mathbf{K} = \int_{\Omega} \left((\mathbf{L} \nabla w)^t [\mathbf{D}] (\mathbf{L} \nabla w) \right) d\Omega$$
where $\mathbf{L} \nabla w = \begin{bmatrix} \frac{\partial \theta_y}{\partial x} \\ \frac{\partial \theta_x}{\partial y} \\ \frac{\partial \theta_x}{\partial y} + \frac{\partial \theta_y}{\partial x} \end{bmatrix} = \begin{bmatrix} -\frac{\partial^2 w}{\partial x^2} \\ \frac{\partial^2 w}{\partial y^2} \\ 2\frac{\partial^2 w}{\partial x \partial y} \end{bmatrix}$

The corresponding finite element formulation is : $\mathcal{E}(\underline{\mathbf{V}}) := \underline{\mathbf{K}}\underline{d} - \underline{\mathbf{F}}\underline{b} = 0$, such as $\underline{b} = (\underline{\mathcal{M}}_n, \underline{\mathcal{V}}_n, \underline{R})$.

omenclature

- vertical displacement of the plate
- transversal load
- the flexural rigidity of the plate
- regularization parameter
- R_i concentrated forces at corner nodes
- *n* outer normal
- $H(\Gamma)$ space of the operator solutions
- δ level of noise